Date: 30 August 2023 Project No: LTC21220-A

Revision: B

Ngai Tahu Property 115 Eureka Street, Aranui Attention: Simon Tucker

RE: SHALLOW SOIL TEST REPORT
FUTURE RESIDENTIAL FOUNDATIONS
LOT 3, 115 EUREKA STREET, ARANUI



### LandTech Consulting Ltd

Email: info@landtech.nz
Postal: PO Box 119, Christchurch 8013

Auckland Office: 09 930 9334 15C Paramount Drive, Henderson

Christchurch Office: 03 390 1371 11B Carlyle Street, Sydenham

www.landtech.nz

# 1.0 Introduction & Background

LandTech Consulting Ltd (LandTech) were engaged by Ngai Tahu Property to carry out lot specific shallow soil testing at Lot 3, 115 Eureka Street, Aranui (the site). The purpose of the shallow soil testing is to confirm subsurface conditions and provide geotechnical recommendations with regards to future residential foundations within the site.

We previously investigated the underlying larger property, which this lot has been formed from, to support the corresponding Resource Consent for the subdivision. Our findings are presented in a report titled: *Geotechnical Investigation Report for Proposed Residential Subdivision 115 Eureka Street, Aranui*, project reference: LTC21220, Revision B, dated 13 September 2021.

LandTech has been retained to monitor earthworks and ground remediation for the subdivision site. As earthworks are ongoing across the wider subdivision site at the time of preparing this letter, we have yet to prepare an Earthworks Completion Report for the subdivision. However, earthworks were mostly completed at this particular lot, with the exception of potentially some additional minor topsoil spreading, prior to our current shallow soil testing and had been carried out to the appropriate engineering standards. This report is considered relevant for the ground conditions beneath the lot at the time of testing, and any additional earthworks (ie topsoil spreading) will need to be taken into account for excavation depths of future house foundations. Depths of foundation excavations presented later within this report may therefore need to be increased at the time of house construction should additional topsoil be placed after the date of our testing.

## 2.0 Shallow Soil Testing

LandTech investigated the site on 8 June 2023, conducting two hand augers with corresponding Scala Penetrometer tests (SPs) and two additional SPs. The locations of the hand auger and SPs were measured using inferred lot boundaries and are approximate only. The test locations are shown on our LandTech *Test Location Plan*, Drawing No. Lot 3/ TLP, attached to this report.

Soil types and conditions within the hand augers were logged by LandTech technical staff in accordance with New Zealand Geotechnical Society *Guideline for the Description of Soil and Rock for Engineering Purposes* (2005). The hand auger logs for each hole are attached.

Our SP testing was carried out in accordance with NZS 4402:1988, Test 6.5.2, *Dynamic Cone Penetrometer*. The SP results are also attached.

Topsoil was encountered within the hand augers from the ground surface to depths between 0.2m to 0.4m, which was typically a dark brown silty sand material with a trace of gravel. Engineered fill was encountered below the topsoil to the base of auger holes, which was a compacted brown sand material with a trace of gravel. The hand augers were terminated at 1.2m depth below ground level to avoid damage to the geotextile layer which was installed at 1.5m depth below ground level. Groundwater was not encountered during testing.

As part of the subdivision earthworks, ground improvement consisting of a reinforced raft is being constructed across the site, which we observed the subgrade excavations for and monitored construction. The reinforced engineered fill raft comprises 250mm compacted sand layers, with a total thickness of 1200mm, capped with approximately 300mm of topsoil. A layer of geotextile (AS410) was installed at the base of the engineered fill raft and three geogrid layers (Duragrid 40/40) were installed within it for reinforcement (at the base, 250mm, and 750mm).

SPs at each of the four locations returned results between 2 and 11 blows per 100mm penetration below the topsoil and within the engineered fill layer. SPs were terminated at 0.9m depth to confirm near surface bearing capacities, and to avoid damage to the geogrids and geotextile. In addition, the density of the engineered fill and bearing capacity of its underlying subgrade have been confirmed via extensive geotechnical inspection and fill compaction testing during subdivision earthworks.



#### 3.0 Foundation Recommendations

From our previous investigation in support of the subdivision, we concluded that TC2 foundations in accordance with MBIE Guidelines (December 2012) may be used at this site following subdivision earthworks and ground remediation (reinforced engineered fill raft). Our current lot-specific shallow soil testing confirms such foundations remain appropriate. In particular, we confirm Option 4 enhanced slab foundations are required.

All foundations must be embedded to a minimum depth of 0.4m below ground level, where an Ultimate Bearing Capacity of 200kPa and a strength reduction factor of (\$\phi\$s) 0.5 is available. Foundations excavations should expose engineered fill of appropriate bearing capacity. All topsoil and unsuitable materials should be removed below foundations and floor slab areas, and where required layers of compacted hardfill should be placed to the underside of foundations.

The external boundaries of the lot to the south-eastern side and south-western side, and internal north-western side by the driveway, are to the rear of a circa 0.6m tall retaining wall, which we understand has not been designed for surcharge loading from future housing within the lots. Should any building foundations, permanent earthworks, or additional retaining walls be constructed within 3.5 x wall height to the rear of the boundary retainer constructed during subdivision works, then these shall be subject to specific engineering design to make sure no undue surcharge is applied to the rear of the existing subdivision retaining wall.

#### 4.0 Limitations

This shallow soil testing report has been prepared for our client, Ngai Tahu Property, to support foundation design and corresponding Building Consent applications for future housing within the corresponding lot. This report shall not be extrapolated for other nearby sites or used for any other purposes without the express approval of LandTech and our client.

The findings of this report are based on the results of testing conducted at point locations and within the agreed scope of works. Therefore, while we have attempted to conduct a thorough investigation of soils across the site, local subsurface conditions could vary from those used in our geotechnical model, as soils can vary naturally and due to previous human activities. LandTech has no control over and should not be held accountable for these variations. Should exposed soil conditions vary from those described herein, we request to be informed so that we may assess the continued applicability of our recommendations.

Our geotechnical investigation described in this report was confined to geotechnical aspects of the site only and did not involve an assessment for environmental contaminants. Our investigation and analyses



have not taken into account possible fault rupture that may cause deformations and displacements of the ground directly below the site. These assessments are outside the scope of our geotechnical engagement.

If you have any queries regarding this report, please contact the undersigned at your convenience.

Yours faithfully,

LandTech Consulting Limited

Prepared By:

Remus Marchis – Engineering Geologist

BSc (geology), PMEG

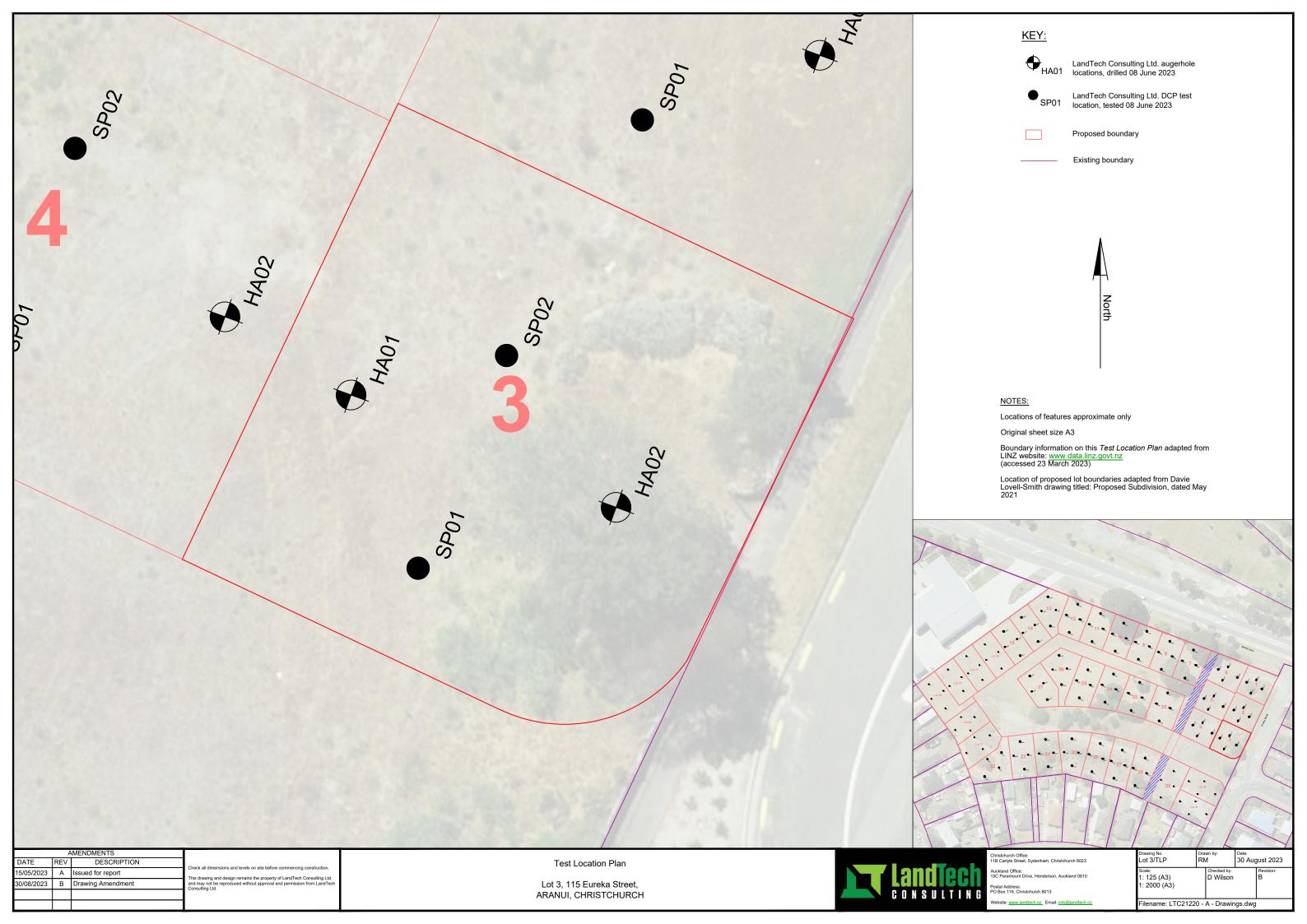
Authorised By: Dwayne Wilson - Director

ne won

CMEngNZ, CPEng, IntPE(NZ)

**Attachments:** Test Location Plan

**Test Results** 





Client: Ngai Tahu Property

Project: Lot Specific Testing

Address: 115 Eureka Street, Aranui, Christchurch

Augerhole No.: Lot 3 - HA01

Sheet No.: 1 of 1

 Project No.:
 LTC21220-A
 Coordinates:
 NZTM2000: E1576434.28, N5182975.19
 Logged By:
 KP

 Drill Type:
 50mm Hand Auger
 Reduced Level:
 2.83m (LVD1937)
 Shear Vane No.:

 Date Started:
 08-Jun-23
 Ground Conditions:
 Near level, Topsoil.
 Calibration Factor:

 Date Finished:
 08-Jun-23
 Groundwater Level (m):
 Not Encountered
 Calibration Date:

		: 08-Jun d: 08-Jun		Ground Conditions: Near level, Topsoil.  Groundwater Level (m): Not Encountered			Calibration Factor: Calibration Date:						
			` ·				In-situ Fiel						
phy	Depth (m)	Graphic Log	Description	m)	Ê	Shear Strength (kPa) Dynamic Cone (Scala) Penetrometer							
Stratigraphy			Soil description in accordance with Guideline for the Field Classification and Description of Soil and Rock for Engineering Purposes, NZ Geolechnical Society Inc, 2005	Groundwater Level (m)	Depth (m)	Peak     Peak	Ê	Tun Scala Ble		ow Count / 0mm			
Str			Engineering Pulposes, NZ Geolechina Society Inc., 2005	ê,	۵	• Remoulded 50 100 150 200	Depth (m)	Blow Count					
=		IS TATE	Silty fine to medium SAND, trace fine to medium subround greywacke gravel,			30 100 130 200			U 3   :	10 15 2			
Topsoil / Fill	_	15 4 4 4 4 4 4 4 4 4 4 4 4 4	dark brown, loose, moist, trace rootlets [TOPSOIL/FILL].		_		-0.1	2					
Copsc		LS " T " T T L LS T					-0.2	3					
	-		Fine to medium SAND, trace fine to medium subround greywacke gravel,	1	-	-			🛉				
	_	$\bowtie$	brown, medium dense to dense, wet, [ENGINEERED FILL].				-0.3	3					
		$\bowtie$					-0.4	3					
	-				-	+		_	🕇				
	0.5 _	$\bowtie$			0.5 _		-0.5	5					
		$\bowtie$					-0.6	7					
≣	-				-	1	0.7		<b>\</b>				
Engineered Fill	-						-0.7	8	<b> </b>				
ingine		$\bowtie$					-0.8	9	\				
ш	-					1	-0.9	8					
	-						0.0						
	4.0	$\bowtie$			4.0								
	1.0 _	$\bowtie$	1.0m: Trace medium to coarse subround greywacke gravel, GeoGrid.		1.0 _	1							
	-				-								
	_		End of Augerhole: 1.2m			]							
	-		[TARGET DEPTH]			-							
	_				_								
	1.5 _				1.5 _	1							
	-					]							
	-				-	1							
	-												
					_								
	2.0 _				2.0 _	1							
	_												
	-				-	1							
	-												
	-					]							
	2.5 _				2.5 _								
	_												
	-					1							
	-					4							
	-				-	1							
				In-situ teeti	ng inaccord	lance with the following standards:							
Target de	epth achi	eved.				esting: NZS 4402:1988, Test 6.5.2, D	ynamic Cone Pene	trometer					

Shear Vane Testing: Guideline for Hand Held Shear Vane Test, NZGS, August 2001



Client: Ngai Tahu Property

Project: Lot Specific Testing

Address: 115 Eureka Street, Aranui, Christchurch

Augerhole No.: Lot 3 - HA02

Sheet No.: 1 of 1

 Project No.:
 LTC21220-A
 Coordinates:
 NZTM2000: E1576446.55, N5182970.01
 Logged By:
 KP

 Drill Type:
 50mm Hand Auger
 Reduced Level:
 2.83m (LVD1937)
 Shear Vane No.:

 Date Started:
 08-Jun-23
 Ground Conditions:
 Near level, Topsoil.
 Calibration Factor:

 Date Finished:
 08-Jun-23
 Groundwater Level (m):
 Not Encountered
 Calibration Date:

Description $\frac{1}{2}$ $\frac{1}{2}$ $\frac{1}{2}$ Shear Strength (kPa) Dynamic Cone (Scala) I	Finished:	<b>l</b> : 08-Jun	n-23 Groundwater Level (m): Not Encour	itered		Calibration Date:						
Silly fine to medium SAND, trace fine to medium subround greywacke gravel, and the town, loose, moist, trace rootlets [TOFSOIL/FILL].	=	go		Groundwater Level (m)	=	In-situ Field Testing  Shear Strength (kBa) Dynamic Cone (Scala) Penetrometer						
1.5					Depth (m	Peak     Remoulded				Scala Blow 100n	Count /	
1.5	# TS # # # # # # # #	S W W W W S S W W W S S W W W S S W W W S S W W S S W W S S S W S	Silty fine to medium SAND, trace fine to medium subround greywacke gravel, dark brown, loose, moist, trace rootlets [TOPSOIL/FILL].		-			-0.1 -0.2	1	•		
The to medium SAND, trace silt and fine to medium subround greywacke gravel, brown, dense, moist, [ENGINEERED FILL].   1.0	一TS、 业、	L TS _ A A A A A A A A A A A A A A A A A A	^0.3m: Some fine to medium subround greywacke gravel.		-							
End of Augerhole: 1.2m  [TARGET DEPTH]  1.5 - 2.0 - 2.		$\times\!\!\times\!\!\times$	Fine to medium SAND, trace silt and fine to medium subround greywacke gravel, brown, dense, moist, [ENGINEERED FILL].		0.5 _							
End of Augerhole: 1.2m [TARGET DEPTH]  1.5 _			``0.6m: Wet.		-	_					•	
End of Augerhole: 1.2m [TARGET DEPTH]  1.5			^0.8m: Trace fine to coarse subround greywacke gravel.									
2.0	1.0				1.0 _	-						
					-							
	1.5				1.5 _							
	-											
2.5_	2.0				2.0 _							
2.5 _					-							
					-							
	2.5				2.5 _							
					-							
					-							

Shear Vane Testing: Guideline for Hand Held Shear Vane Test, NZGS, August 2001



Client: Ngai Tahu Property

Project: Lot Specific Testing

Address: 115 Eureka Street, Aranui, Christchurch

Lot 3 - SP01 Lot 3 - SP02 Tested By: KP Tested By: ΚP Tested By: Tested By: Ground Conditions: Near level, Topsoil. Ground Conditions: Near level, Topsoil. **Ground Conditions: Ground Conditions:** Coordinates: NZTM2000 E 1576436.93 Coordinates: NZTM2000 E 1576440.58 Coordinates: Coordinates: N 5182968.24 N 5182976.73 Test Date: Test Date: 08-Jun-23 Test Date: 08-Jun-23 Test Date: DEPTH (m) DEPTH (m) DEPTH (m) **SCALA PENETROMETER** SCALA PENETROMETER **SCALA PENETROMETER SCALA PENETROMETER** DATA DATA DATA DATA (Blows / 100mm) (Blows / 100mm) (Blows / 100mm) (Blows / 100mm) 0.2 0.2 0.2 2 5 0.4 0.4 0.4 5 6 0.6 0.6 8 0.8 8.0 0.8 9 1.4 1.6 2.0 2.0 2.2 2.2 2.4 2.4 2.6 2.6 2.6 2.8 2.8 2.8